



# Improving Design Phase

# Evaluations for High Pile Rebound Sites



## *The Mysterious Case of the Bouncing Piles*

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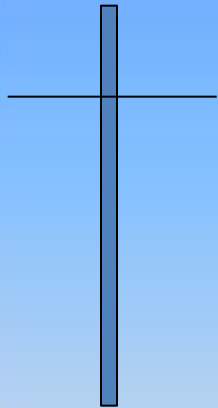
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# Defining the Problem

## **Excessive Pile Rebound and/or Bouncing**

- ❖ High Displacement Piles
- ❖ Typically driven by Diesel Hammers
- ❖ Cause Dilation of Very Dense Saturated Silty Sands to Sandy Silts

## **Capacities & Depths not achieved**



Pile  
Rebound  
*Old School  
Recording*

Courtesy  
of GRL  
Library





# Digital Record of Rebound from PDA sensors

*Displacement (inches)*

D ( 1. 00)

*DMX = max displacement*

**0.75 in**

**Rebound = DMX - SET = 0.725 in**

SET = 0.0255319 in

TS: 204.8  
TB: 0.0

**PDA Recording Time (milliseconds)**

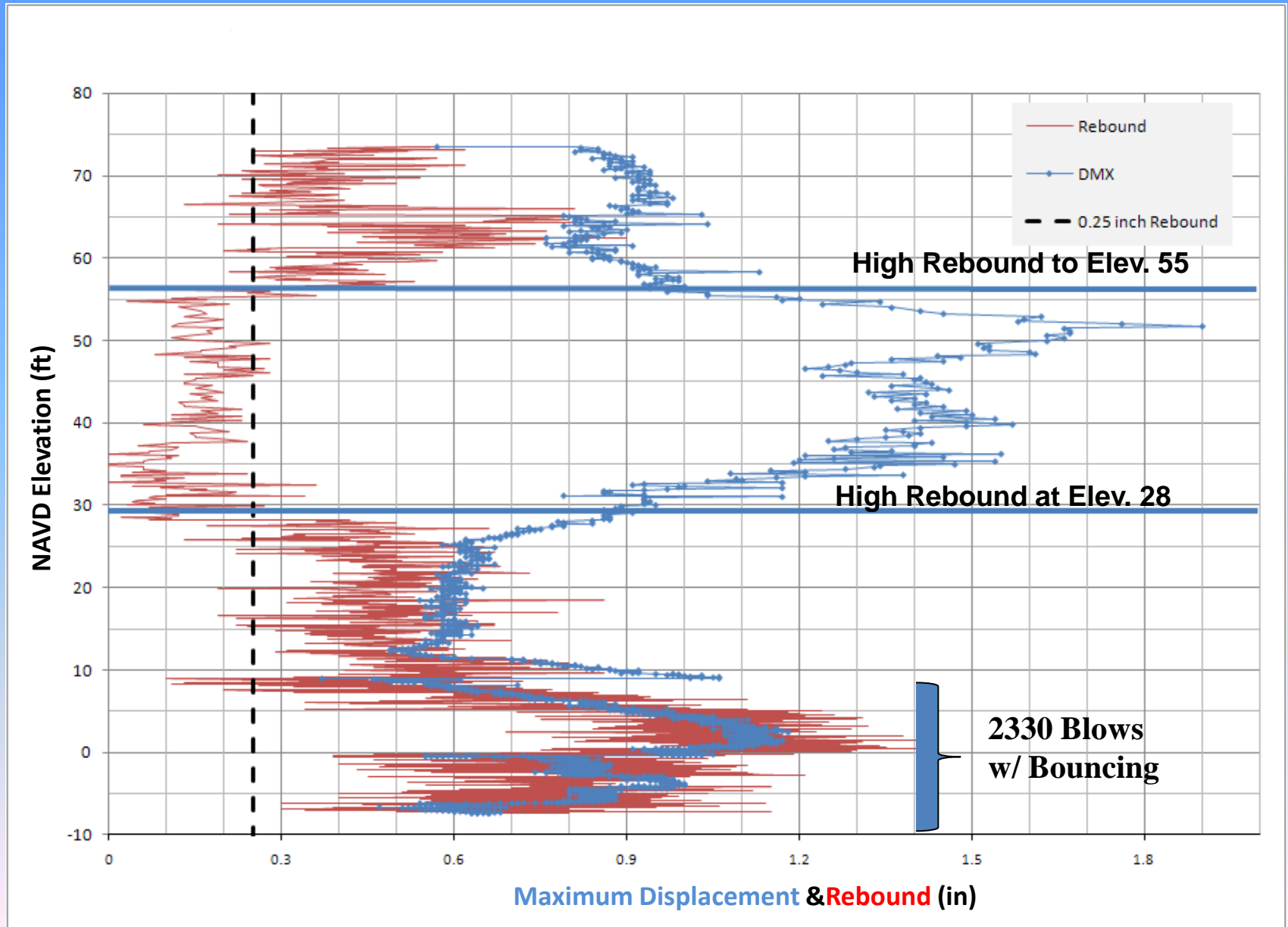


# Phase I

- 🐾 Retested Soils at 3 sites with PDA data
  - ❖ I-4/SR 408 Anderson Street Overpass
  - ❖ I-4/John Young Parkway
  - ❖ Ramsey Branch Bridge SR 83 US 331 over Choctawhatchee Bay: District 3
- 🐾 SPT, CPT, PMT, DMT, PocketPen
- 🐾 Shelby Tubes
  - ❖ CU Triaxial, Permeability etc.



# Anderson Street PDA vs. Elevation





## Silt Content Summary

<b>Site Name</b>	<b>Rebound Soil Silt Content (%)</b>	<b>Increase in Silt Content (%)</b>
Anderson Street	19	40
John Young	17	20
Ramsey Branch	20	100



## *N Values Summary*

Site Name	N In Rebound Soil (blows/ft)	Increase in N In Rebound Soil (%)
Anderson St	27	290
John Young	16	220
Ramsey Branch	7	600



## *Pocket Penetrometer $q_u$ , & CPT $q_c$ , $f_s$ Summary*

Site Name	Pocket Penetrometer Increase in $q_u$ In Rebound Soil (%)	CPT Increase in $q_c$ In Rebound Soil (%)	CPT Increase in $f_s$ In Rebound Soil (%)
Anderson St	260	100	250
John Young	200	0*	-10*
Ramsey Branch	40	750	780

\* John Young Site more layered



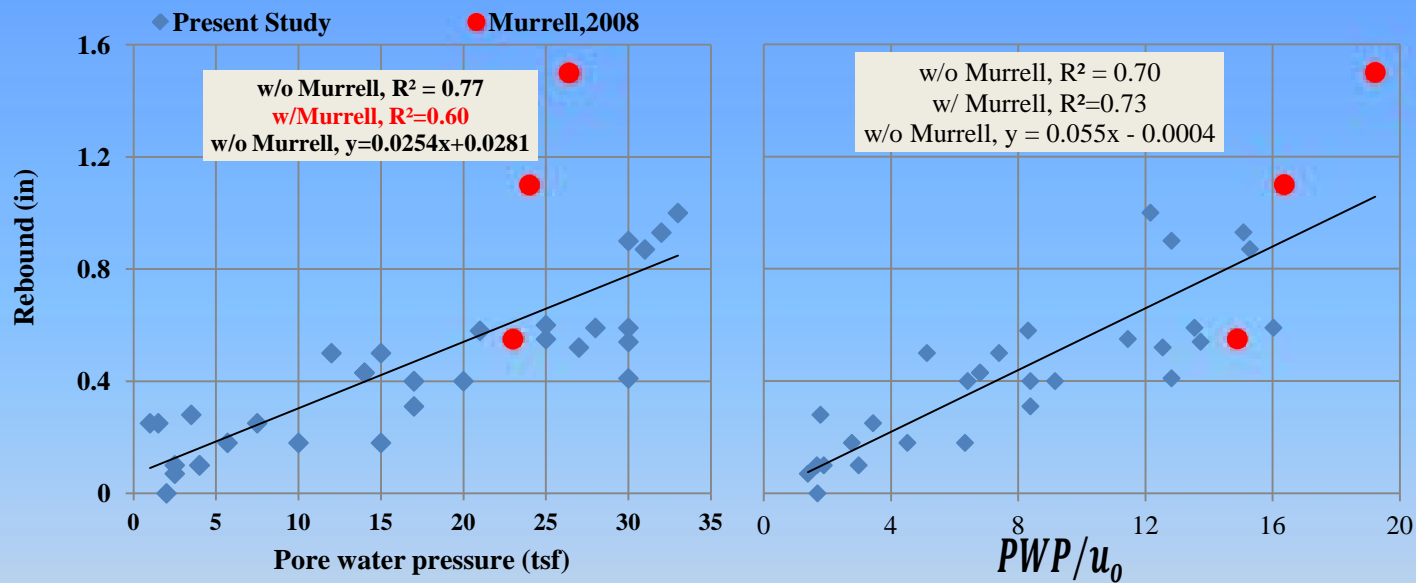
# Conclusions

- ❖ Parameters that generally increased
  - ❖ Silt Content
  - ❖ SPT N values
  - ❖ Pocket Penetrometer  $q_u$
  - ❖ CPT  $q_c$  and  $f_s$
- ❖ PDA displacement vs. elevation shows rebound zones.
- ❖ Bouncing elevations match elevation of the dense or very dense to hard silty sands and clays (SM/SC/CH)



# FIT Unfunded Findings

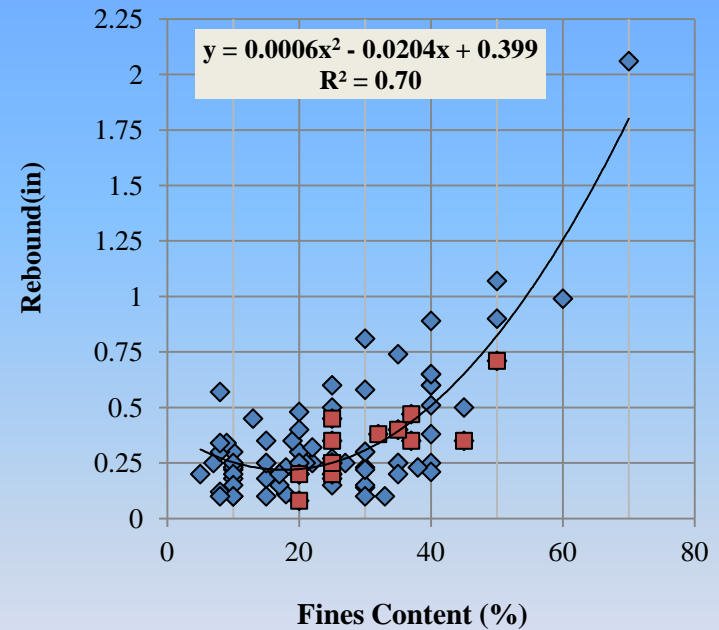
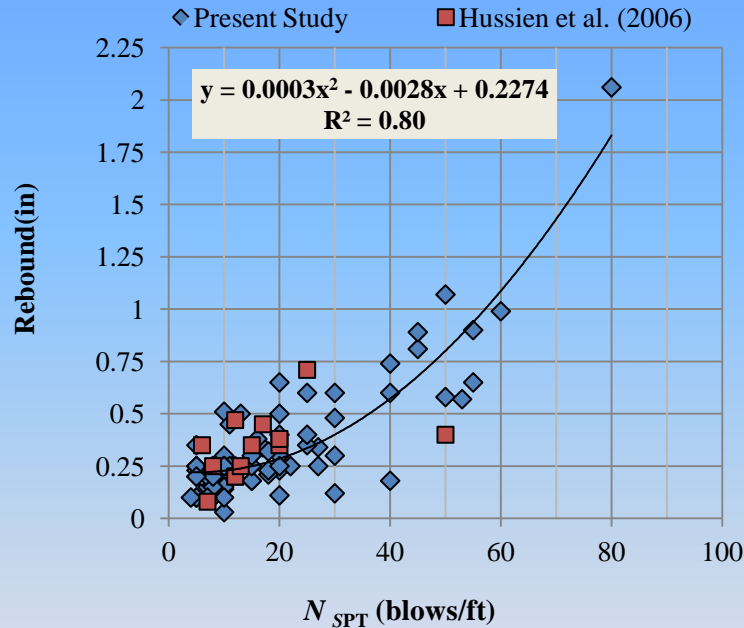
## CPTu Pore Water Pressure Correlations





# FIT Unfunded Findings

## Rebound vs N and Fines Content





# Phase II Research Objective

 Prove these correlations are reliable



# Project Schedule

RESEARCH TASK
Task 1 <b>Literature Search</b>
Task 2 <b>Develop Locations of New Testing Sites</b>
Task 3 <b>Test Program for New Field Testing Sites</b>
Task 4 <b>Field Data Reduction</b>
Task 5 <b>Laboratory Testing and Reduction of Disturbed Samples</b>
Task 6 <b>Laboratory Testing, Reduction and Analysis of Thin Walled Tube Samples</b>
Task 7 <b>Analyze Reduced Laboratory Data from Disturbed Testing</b>
Task 8 <b>Analyze Reduced Field Data</b>
Task 9 <b>Technology Transfer for Reporting and Presentations</b>



# Cyclic Testing



# Cyclic Objective

🐿 Determine and evaluate the variations in cyclic pore water pressure during cyclic triaxial testing for high pile rebound soils.

The study will focus on :

- ✓ Evaluation of pore water pressure generation during cyclic loading in and above rebound zone
- ✓ Influence of fines and silt content on pore water pressure generation during cyclic loading



# Testing Program:

## Identify Site Locations

Case	Site Name	Rebound Observed?
1	I 4 and US 192, Orlando, Florida	Excessive Rebound
2	I-4/Osceola Parkway, Central Florida, Osceola County	
3	I 10 and Chaffee Road, Jacksonville, FL	
4	SR 417/International Parkway, Seminole County, FL	Non-Rebound
5	I-4/SR 408 (Ramp B), Orange County, Florida	



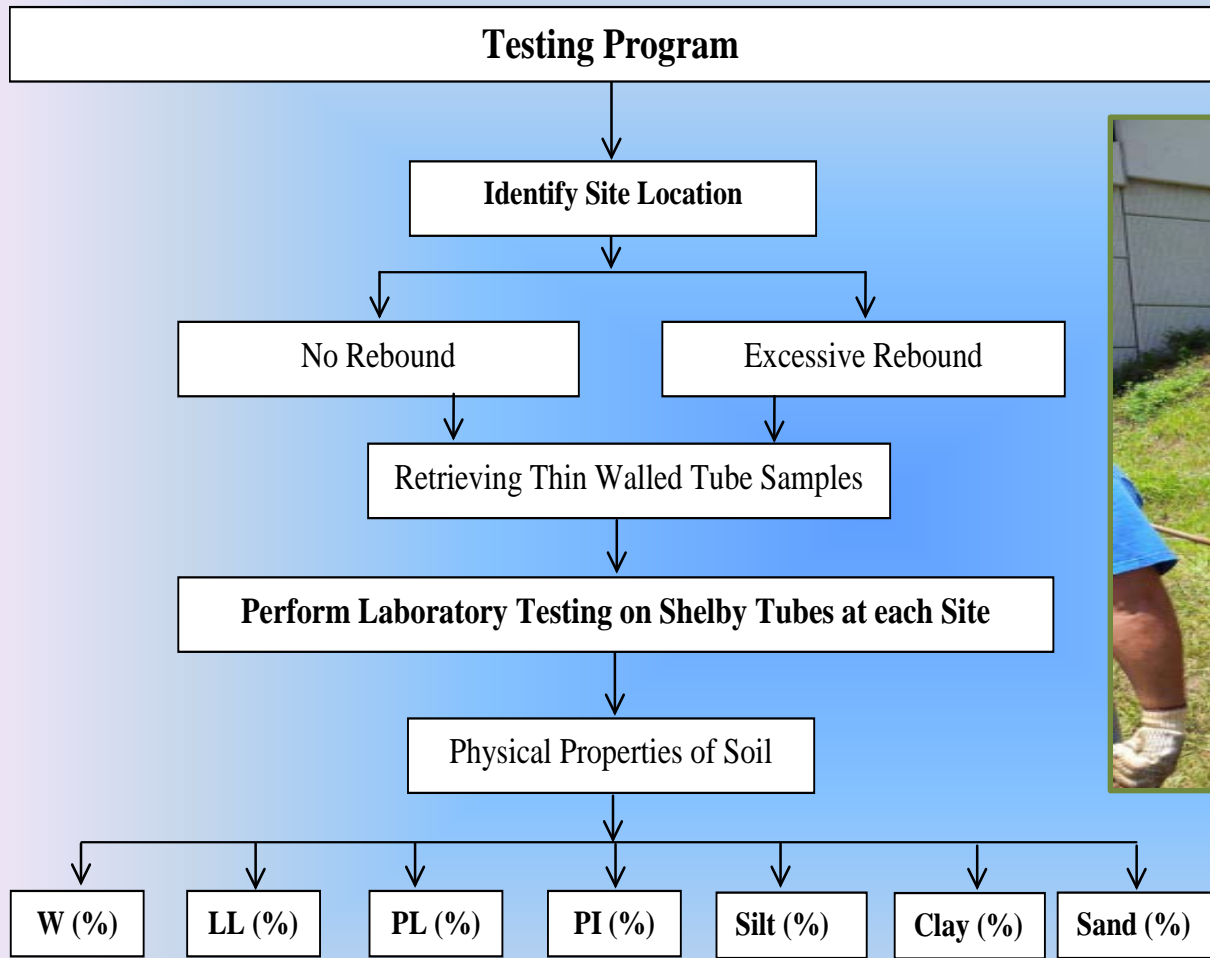
I 4 and US 192, Orlando, Florida

Elve.	Soil Profile	Pore Water Pressure (tsf)	Shelby Tube Sample	Rebound Zone
GSE + 144.31 ft				
88.3 ft				
72 ft	Brown Fine Sand with Silt (SP-SM)	Less than 3 tsf	1	No Rebound
67 ft				
57 ft	Brown Fine Sand with Silt (SP-SM)	Less than 5 tsf	1	High Rebound (0.28 to 0.5 in)
53 ft	Light Brown Fine sand (SP)			
	Light Brown Sand with Silt (SP-SM)			
	Light Brown Fine sand (SP)			
	Gray Fine Sand with Silt (SP-SM)			
	Gray Silty Fine Sand (SM)			
23 ft				
19 ft	Gray Silty Fine Sand (SM)	More than 9 tsf	1	High Rebound (0.26 to .82 in)
	Green Clayey Fine Sand (SC)			
	Green Sandy Silt (ML)			
0 ft	Green Clayey Fine Sand (SC)			
	Tan Highly Weathered Limestone			
	Bt at 193'			

Westbound: End Bent 1

- Rebound Zone
- Shelby Tube Location

<sup>a</sup> Shelby Tube Location  
 ▾ Groundwater Elevation  
 BT Boring Terminated





# Cyclic Testing

Consolidated Undrained (CU) Triaxial Test

Deviator stress at failure

Loading Stages

Applied Deviator Stresses

$$\Delta\sigma_v = 0.1 \times \Delta\sigma_f$$

$$\Delta\sigma_v = 0.2 \times \Delta\sigma_f$$

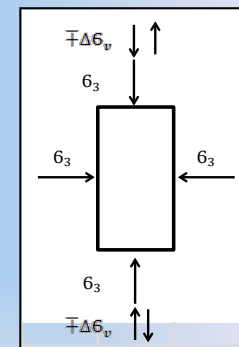
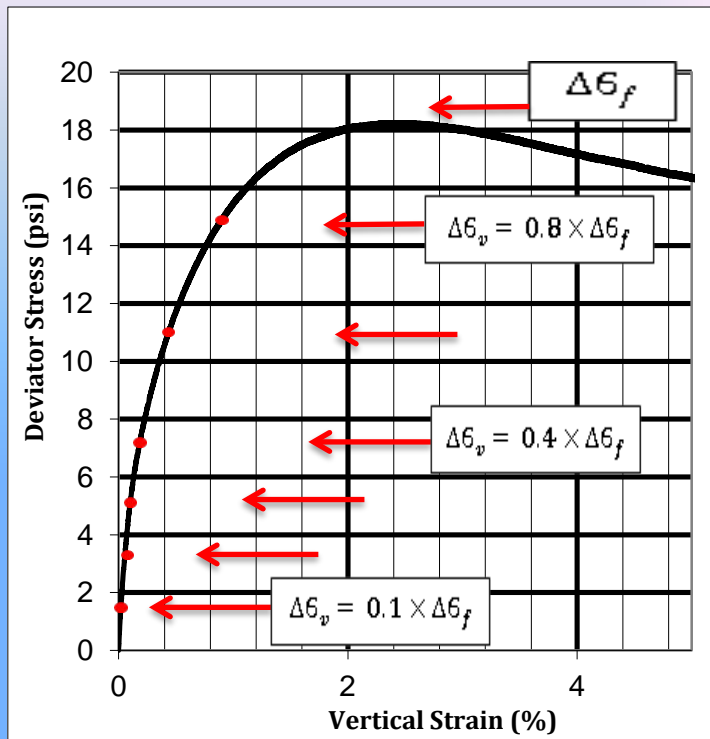
$$\Delta\sigma_v = 0.3 \times \Delta\sigma_f$$

$$\Delta\sigma_v = 0.4 \times \Delta\sigma_f$$

$$\Delta\sigma_v = 0.6 \times \Delta\sigma_f$$

$$\Delta\sigma_v = 0.8 \times \Delta\sigma_f$$

1000 cycles at each loading stage





# Questions